

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

Y. SHEN and B.M. GIBBS

DEPARTMENT OF BUILDING ENGINEERING, UNIVERSITY OF LIVERPOOL.

1. Introduction

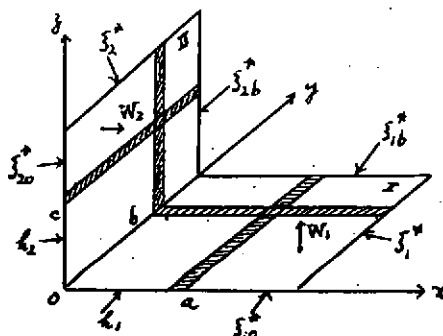
Statistical energy analysis (SEA) methods are often used in the calculation of power flows between coupled structural elements and the vibration levels of each element of a system. The method gives only the relationships between average quantities and the results are usually unreliable in the low frequency range where model densities are low; a deterministic solution is therefore preferred although the calculations are more complicated if not intractable.

This paper describes an approximate method used to calculate the bending vibrations of a combination of rectangular thin plates with elastically supported and damped non-coupled edges subjected to point sinusoidal excitations. The method has clear applications in the prediction and control of machine induced vibrations in buildings or in any case where low frequency vibration sources are coupled to a system of beams and plates. The method is similar to that used for calculating the bending vibrations of a single rectangular plate where the displacement amplitude function is expressed as a linear combination of coordinate functions and is extended to the case of combination of plates. The displacement amplitude function vector of the global system of plates is also expressed as a linear combination of coordinate function vectors which satisfy all the boundary conditions of the global system.

Two examples, an L-combination and a series of T-combinations of rectangular thin plates, are discussed and the input and transfer mobilities derived. The frequency response of combinations of concrete plates is calculated in the frequency range below 500 Hz, and the effect of material damping and edge losses is investigated.

2. L-combination of two rectangular plates

2.1 Differential equations



The governing differential equations of bending vibration of the L-combination of rectangular thin plates (as shown in Fig.1) can be written as

Fig.1.

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

$$\begin{pmatrix} L_1 & 0 \\ 0 & L_2 \end{pmatrix} \begin{pmatrix} w_1(x, y, z) \\ w_2(y, z, z) \end{pmatrix} = \begin{pmatrix} \hat{f}_1(x, y, z) \\ \hat{f}_2(y, z, z) \end{pmatrix} \quad (1)$$

where $L_i \equiv D_i^* \nabla_i^4 - \rho_i \hat{f}_i \frac{\partial^4}{\partial t^4}$, $i = 1, 2$, $\nabla_1^4 = \frac{\partial^4}{\partial x^4} + 2 \frac{\partial^2}{\partial x^2} \frac{\partial^2}{\partial y^2} + \frac{\partial^4}{\partial y^4}$,

$$\nabla_2^4 = \frac{\partial^4}{\partial y^4} + 2 \frac{\partial^2}{\partial y^2} \frac{\partial^2}{\partial z^2} + \frac{\partial^4}{\partial z^4}.$$

$\rho_i, \hat{f}_i, D_i^* (-D_i^* (1+j\eta))$ are respectively the density, thickness and complex bending stiffness of the i th plate. $(w_1(x, y, z))$ and $(\hat{f}_1(x, y, z))$ are displacement function and pressure function vectors. $(w_2(y, z, z))$ and $(\hat{f}_2(y, z, z))$ are displacement function and pressure function vectors. If the excitations are sinusoidal i.e.

$$\begin{pmatrix} \hat{f}_1(x, y, z) \\ \hat{f}_2(y, z, z) \end{pmatrix} = \begin{pmatrix} Q_1(x, y) \\ Q_2(y, z) \end{pmatrix} e^{j\omega t} \quad (2)$$

where ω is the radian frequency and $\begin{pmatrix} Q_1(x, y) \\ Q_2(y, z) \end{pmatrix}$ is the pressure amplitude function vector applied to the global system, then equation (1) reduces to

$$\begin{pmatrix} L_1 & 0 \\ 0 & L_2 \end{pmatrix} \begin{pmatrix} W_1(x, y) \\ W_2(y, z) \end{pmatrix} = \begin{pmatrix} Q_1(x, y) \\ Q_2(y, z) \end{pmatrix} \quad (3)$$

where $L_i \equiv D_i^* \nabla_i^4 - \omega^2 \rho_i \hat{f}_i$, $i = 1, 2$ and $\begin{pmatrix} W_1(x, y) \\ W_2(y, z) \end{pmatrix}$ is the bending displacement amplitude function vector.

2.2 Boundary conditions

The boundary conditions can be written as:

On $y = 0, z = 0$ edge;

$$\left. \begin{aligned} W_1(x, 0) &= 0 \\ D_1^* \left(\frac{\partial^2 W_1}{\partial y^2} + \rho_1 \frac{\partial^2 W_1}{\partial x^2} \right)_{y=0} &= \int_{10}^* \left(\frac{\partial W_1}{\partial y} \right)_{y=0} \\ \text{On } y=b, z=0 \text{ edge;} \\ W_1(x, b) &= 0 \\ D_1^* \left(\frac{\partial^2 W_1}{\partial y^2} + \rho_1 \frac{\partial^2 W_1}{\partial x^2} \right)_{y=b} &= - \int_{1b}^* \left(\frac{\partial W_1}{\partial y} \right)_{y=b} \end{aligned} \right\} \quad (4)$$

On $x=a, z=0$ edge;

$$\left. \begin{aligned} W_1(a, z) &= 0 \\ D_1^* \left(\frac{\partial^2 W_1}{\partial x^2} + \rho_1 \frac{\partial^2 W_1}{\partial y^2} \right)_{x=a} &= - \int_{1a}^* \left(\frac{\partial W_1}{\partial x} \right)_{x=a} \end{aligned} \right\} \quad (5)$$

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

On $x=0, y=0$ edge:

$$\left. \begin{aligned} W_1(0, y) = 0, \quad W_2(y, 0) = 0 \\ \frac{\partial W_1(x, y)}{\partial x} \Big|_{x=0} + \frac{\partial W_2(y, 0)}{\partial y} \Big|_{y=0} = 0 \\ D_1^* \left(\frac{\partial^2 W_1}{\partial x^2} + \nu_1 \frac{\partial^2 W_1}{\partial y^2} \right)_{x=0} = D_2^* \left(\frac{\partial^2 W_2}{\partial y^2} + \nu_2 \frac{\partial^2 W_2}{\partial x^2} \right)_{y=0} \end{aligned} \right\} \quad (6)$$

On $x=0, y=b$ edge:

$$\left. \begin{aligned} W_2(0, b) = 0, \quad D_2^* \left(\frac{\partial^2 W_2}{\partial y^2} + \nu_2 \frac{\partial^2 W_2}{\partial x^2} \right)_{y=b} = J_{2c}^* \left(\frac{\partial W_2}{\partial y} \right)_{y=b} \\ W_1(b, y) = 0, \quad D_1^* \left(\frac{\partial^2 W_1}{\partial x^2} + \nu_1 \frac{\partial^2 W_1}{\partial y^2} \right)_{x=b} = -J_{1b}^* \left(\frac{\partial W_1}{\partial x} \right)_{x=b} \end{aligned} \right\} \quad (7)$$

On $x=0, y=c$ edge:

$$\left. \begin{aligned} W_1(y, c) = 0, \\ D_1^* \left(\frac{\partial^2 W_1}{\partial x^2} + \nu_1 \frac{\partial^2 W_1}{\partial y^2} \right)_{y=c} = -J_{1c}^* \left(\frac{\partial W_1}{\partial y} \right)_{y=c} \end{aligned} \right\} \quad (8)$$

$\nu_1, J_{1c}^*, J_{1b}^*, J_{2c}^*$ are respectively the Poisson's ratio and the boundary complex rotational stiffnesses (as shown in Fig.1) of the i th plate, $i = 1, 2$.

2.3 Coordinate function vectors

Let $K_{1\ell\ell}, K_{2\ell\ell}, \bar{\Psi}_{1\ell\ell}(x), \bar{\Psi}_{2\ell\ell}(y)$ be the ℓ th eigen wave numbers and eigen functions of the L-combination of beams of unit width perpendicular to the coupled edge (the shaded areas in Fig.1) which satisfy the boundary conditions corresponding to equations (5), (6) and (8).

Let $K_{1ym}, K_{2ym}, \bar{\Psi}_{1ym}(y), \bar{\Psi}_{2ym}(x)$ be respectively the m th eigen wave numbers, eigen functions of the single beams of unit width parallel to the coupled edge, lying in Plate I and Plate II and satisfying the boundary conditions corresponding to equations (4) and (7). For simplicity, assume $J_{1c}^*/D_1^* = J_{2c}^*/D_2^*$ and $J_{1b}^*/D_1^* = J_{2b}^*/D_2^*$ then the two single beams have same eigen wave numbers and eigen functions.

$$K_{1ym} = K_{2ym} = K_{ym}, \quad \bar{\Psi}_{1ym}(y) = \bar{\Psi}_{2ym}(x) = \bar{\Psi}_{ym}(x), \quad m = 1, \dots, M.$$

The (ℓ, m, n) th coordinate function vector for the global system can be written as:

$$\begin{pmatrix} W_{1\ell m}(x, y) \\ W_{2n\ell}(y, x) \end{pmatrix} = \begin{pmatrix} \bar{\Psi}_{1\ell\ell}(x) \bar{\Psi}_{ym}(y) \\ \bar{\Psi}_{ym}(y) \bar{\Psi}_{2\ell\ell}(x) \end{pmatrix} \quad (9)$$

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

satisfying the boundary conditions on the non-coupled edges. In order to satisfy the boundary conditions on the coupled edge, $m = n$, and the coordinate function vector (9) reduces to:

$$\begin{pmatrix} W_{12m}(x, y) \\ W_{2m2}(y, z) \end{pmatrix} = \begin{pmatrix} \bar{\Psi}_{12}(x) \bar{\Phi}_{ym}(y) \\ \bar{\Phi}_{ym}(y) \bar{\Psi}_{22}(z) \end{pmatrix} \quad (10)$$

2.4 Approximate solution

The approximate solution to equation (3) can be written as:

$$\begin{pmatrix} W_1(x, y) \\ W_2(y, z) \end{pmatrix} = \sum_{l=1}^L \sum_{m=1}^M A_{lm} \begin{pmatrix} \bar{\Psi}_{12}(x) \bar{\Phi}_{ym}(y) \\ \bar{\Phi}_{ym}(y) \bar{\Psi}_{22}(z) \end{pmatrix} \quad (11)$$

Substituting (11) into (3) yields.

$$\begin{pmatrix} \mathcal{L}_1 & 0 \\ 0 & \mathcal{L}_2 \end{pmatrix} \sum_l \sum_m \begin{pmatrix} \bar{\Psi}_{12}(x) \bar{\Phi}_{ym}(y) \\ \bar{\Phi}_{ym}(y) \bar{\Psi}_{22}(z) \end{pmatrix} A_{lm} = \begin{pmatrix} Q_1(x, y) \\ Q_2(y, z) \end{pmatrix} + \begin{pmatrix} \epsilon_1(x, y) \\ \epsilon_2(y, z) \end{pmatrix} \quad (12)$$

where $\begin{pmatrix} \epsilon_1(x, y) \\ \epsilon_2(y, z) \end{pmatrix}$ is an error function vector.

Multiplying both sides of equation (12) by the operator vector $\bar{V}_{l'm'}$.

$$\bar{V}_{l'm'} = \left(\int_0^a \int_0^b \bar{\Psi}_{12}^*(x) \bar{\Phi}_{ym}^*(y) dx dy, \int_0^c \int_0^b \bar{\Phi}_{ym}^*(y) \bar{\Psi}_{22}^*(z) dy dz \right)$$

where $\bar{\Psi}_{12}^*$, $\bar{\Phi}_{ym}^*$, $\bar{\Psi}_{22}^*$ are respectively the complex conjugate functions of $\bar{\Psi}_{12}$, $\bar{\Phi}_{ym}$, $\bar{\Psi}_{22}$ and applying Galerkin's orthogonality condition for the global system $\bar{V}_{l'm'} \cdot \begin{pmatrix} \epsilon_1(x, y) \\ \epsilon_2(y, z) \end{pmatrix} = 0$, yields

$$\sum_{l=1}^L \sum_{m=1}^M T(l', m', l, m) A_{lm} = F_{l'm'} \quad (13)$$

where $T(l', m', l, m) = \bar{V}_{l'm'} \cdot \begin{pmatrix} \mathcal{L}_1 & 0 \\ 0 & \mathcal{L}_2 \end{pmatrix} \begin{pmatrix} \bar{\Psi}_{12}(x) \bar{\Phi}_{ym}(y) \\ \bar{\Phi}_{ym}(y) \bar{\Psi}_{22}(z) \end{pmatrix}$

$$= \int_0^a \int_0^b \bar{\Psi}_{12}^* \bar{\Phi}_{ym}^* \mathcal{L}_1 \bar{\Psi}_{12} \bar{\Phi}_{ym} dx dy + \int_0^c \int_0^b \bar{\Phi}_{ym}^* \bar{\Psi}_{22}^* \mathcal{L}_2 \bar{\Phi}_{ym} \bar{\Psi}_{22} dy dz, \quad (14)$$

$$F_{l'm'} = \bar{V}_{l'm'} \cdot \begin{pmatrix} Q_1(x, y) \\ Q_2(y, z) \end{pmatrix} = \int_0^a \int_0^b \bar{\Psi}_{12}^* \bar{\Phi}_{ym}^* Q_1 dx dy + \int_0^c \int_0^b \bar{\Phi}_{ym}^* \bar{\Psi}_{22}^* Q_2 dy dz, \quad (15)$$

$$l' = 1, \dots, L; m' = 1, \dots, M.$$

Equation (13) can be written in matrix form

$$\begin{pmatrix} T(1, 1, 1, 1) & \dots & T(1, 1, L, M) \\ \vdots & & \vdots \\ T(L, M, 1, 1) & \dots & T(L, M, L, M) \end{pmatrix} \begin{pmatrix} A_{11} \\ \vdots \\ A_{LM} \end{pmatrix} = \begin{pmatrix} F_{11} \\ \vdots \\ F_{LM} \end{pmatrix} \quad (16)$$

$\begin{pmatrix} A \\ \vdots \\ A_{LM} \end{pmatrix}$ and $\begin{pmatrix} F_{11} \\ \vdots \\ F_{LM} \end{pmatrix}$ being generalised coordinate and generalised force vectors.

A_{lm} 's can be solved from equation (16) and by substituting A_{lm} 's into equation (11) one gets the solution to equation (3). The complex eigen frequencies and eigen vectors of the global system can also be calculated from equation (16) letting the generalized force vector equal to zero.

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

Let (S) be the inverse matrix of the (T) matrix in equation (16) and $S(l, m, l', m')$ be the terms of (S) matrix, then equation (16) can be written as:

$$\begin{pmatrix} A_n \\ \vdots \\ A_{LM} \end{pmatrix} = (S) \begin{pmatrix} F_n \\ \vdots \\ F_{LM} \end{pmatrix} \quad (17)$$

2.5 Response to point excitation

If a point sinusoidal exciting force $F_p e^{j\omega t}$ is applied at $P(x_p, y_p)$ in the first plane, then

$$\begin{pmatrix} Q_1(x, y) \\ Q_2(y, y) \end{pmatrix} = \begin{pmatrix} F_p \delta(x-x_p) \delta(y-y_p) \\ 0 \end{pmatrix}$$

$$\Rightarrow F_{l'm'} = V_{l'm'} \cdot \begin{pmatrix} F_p \delta(x-x_p) \delta(y-y_p) \\ 0 \end{pmatrix} = F_p \tilde{\Psi}_{lxl'}(x_p) \tilde{\Phi}_{ym'}(y_p);$$

$$l' = 1, \dots, L, \quad m' = 1, \dots, M.$$

$$\Rightarrow A_{lm} = F_p \sum_{l'=1}^L \sum_{m'=1}^M S(l, m, l', m') \tilde{\Psi}_{lxl'}(x_p) \tilde{\Phi}_{ym'}(y_p)$$

$$\Rightarrow \begin{pmatrix} W_1(x, y) \\ W_2(y, y) \end{pmatrix} = F_p \sum_{l, m, l', m'} S(l, m, l', m') \tilde{\Psi}_{lxl'}(x_p) \tilde{\Phi}_{ym'}(y_p) \begin{pmatrix} \bar{\Psi}_{12l}(x) \bar{\Phi}_{jm}(y) \\ \bar{\Phi}_{ym}(y) \bar{\Psi}_{23l}(y) \end{pmatrix} \quad (18)$$

If a receiving point R is at $R(y_R, \delta_R)$ in the second plane, then the displacement amplitude at R

$$W_2(y_R, \delta_R) = F_p \sum_{l, m, l', m'} S(l, m, l', m') \tilde{\Psi}_{lxl'}(x_p) \tilde{\Phi}_{ym'}(y_p) \bar{\Phi}_{ym}(y_R) \bar{\Psi}_{23l}(\delta_R) \quad (19)$$

The transfer mobility from P to R

$$\begin{aligned} M(1, x_p, y_p; 2, y_R, \delta_R) &= \frac{j\omega W_2(y_R, \delta_R)}{F_p} \\ &= j\omega \sum_{l, m, l', m'} S(l, m, l', m') \tilde{\Psi}_{lxl'}(x_p) \tilde{\Phi}_{ym'}(y_p) \bar{\Phi}_{ym}(y_R) \bar{\Psi}_{23l}(\delta_R) \end{aligned} \quad (20)$$

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

If R coincides with P, then the input mobility at P

$$M(l, x_p, y_p) = j\omega \sum_{l, m, l', m'} S(l, m, l', m') \tilde{\Psi}_{l'l'}(x_p) \tilde{\Phi}_{j'm'}(y_p) \tilde{\Phi}_{j'm}(y_p) \tilde{\Psi}_{ll'}(x_p) \\ = 1/Z(l, x_p, y_p) \quad (21)$$

where $Z(1, \chi_p, y_p)$ is the input impedance at $P(\chi_p, y_p)$ in plane I,

3. A series of T-combination of planes

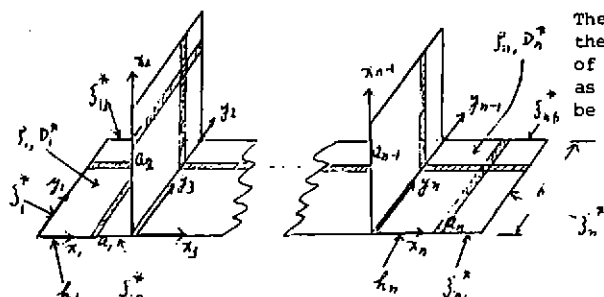


Fig. 2.

The bending vibration of the global system of a series of T-combinations of n planes as shown in Fig.2 can also be solved in the same way as

discussed in Section 2 for the L-combination. The coordinate function vectors are organized from the eigen modes of the series of T-combinations of beams of unit width and the eigen modes of single beams parallel to the coupled edges (the shaded areas in Fig.2.).

4. Frequency response curves due to point excitation

The frequency response curves of an L-combination of concrete plates (as shown in Fig.1) due to unit force excitation at (3,1) in Plate 1 are given in Fig.3 (a), (b), (c). The constants of plates and boundary rotational stiffnesses are (all in S.I. Units):

$$a=5, b=4, c=3; \quad \mathbf{A}_1 = \mathbf{A}_2 = 0.1; \quad \mathbf{B}_1^* = \mathbf{B}_2^* = 2.2 \times 10^6 (1+j2);$$

$$\frac{f_1 k_1}{D_1^*} = \frac{f_2 k_2}{D_2^*} = 1.03 \times 10^{-4} (1 - f_2); \quad (f_2 = 1\%, 5\%, 10\%);$$

Proceedings of The Institute of Acoustics

FLEXURAL VIBRATION TRANSMISSION AT JUNCTIONS

all boundary rotational stiffnesses $= 100(1+j^{20}\%)$. Receiving points are R_1 (2, 2.5) in plate I, R_2 (2,2) and R_3 (0.15, 0.15) in Plate II.

The frequency response curves of a series of T-combinations (as shown in Fig.2) of five concrete plates due to unit force excitation at (2.1, 1.5) in Plate I are given in Fig.4 (a), (b), (c). The constants of plates and boundary rotational stiffnesses are (all in S.I. units):

$$u_1 = 3.0, u_2 = 3.3, u_3 = 4.5, u_4 = 4.3, u_5 = 3.5, b = 4.0;$$

$$k_1 = k_2 = k_3 = k_5 = 0.1, k_4 = .071;$$

$$D_1^* = D_2^* = D_3^* = D_5^* = 2.2 \times 10^6 (1+j^7\%), D_4^* = 0.778 \times 10^6 (1+j^{10}\%);$$

$$\frac{I_1 k_1}{D_1^*} = \frac{I_2 k_2}{D_2^*} = \frac{I_3 k_3}{D_3^*} = \frac{I_5 k_5}{D_5^*} = 1.03 \times 10^{-4} (1+j^7\%), \frac{I_4 k_4}{D_4^*} = 2.06 \times 10^{-4} (1+j^{10}\%);$$

$$J_1^* = 10^5 (1+j^7\%), J_2^* = J_3^* = 2 \times 10^5 (1+j^7\%), J_4^* = 0.5 \times 10^5 (1+j^{10}\%);$$

$$J_{16}^* = J_{20}^* = J_{20}^* = J_{20}^* = 1.1 \times 10^5 (1+j^7\%), J_{40}^* = .387 \times 10^5 (1+j^{10}\%);$$

$$J_{16}^* = J_{20}^* = J_{20}^* = J_{20}^* = 2.2 \times 10^5 (1+j^7\%), J_{40}^* = 0.778 \times 10^5 (1+j^{10}\%);$$

($\gamma = 1\%, 5\%, 10\%$). Receiving points R_2 (3.1, 3.2), R_3 (2.5, 1.8) and R_5 (1.8, 2.3) are in plate II, plate III and plate V respectively.

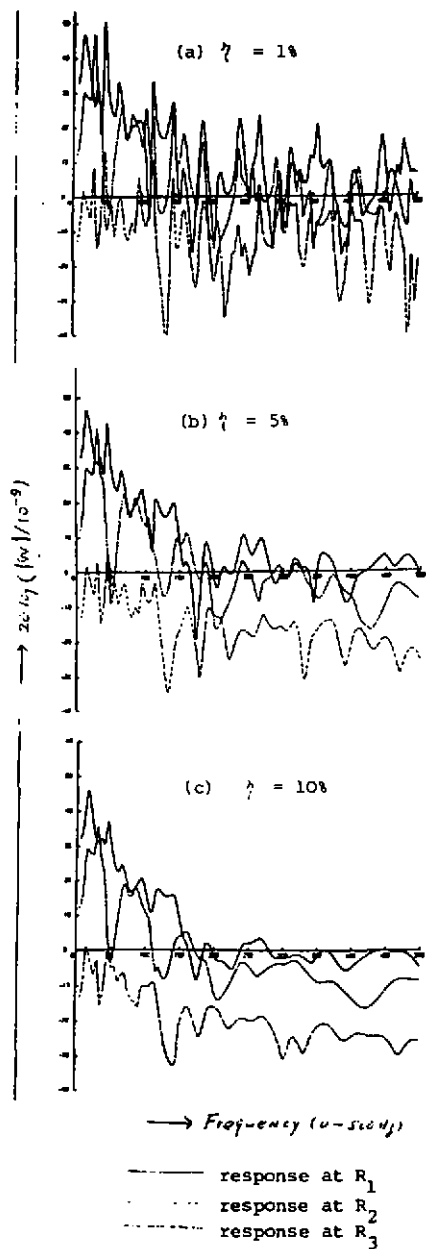


Fig.3

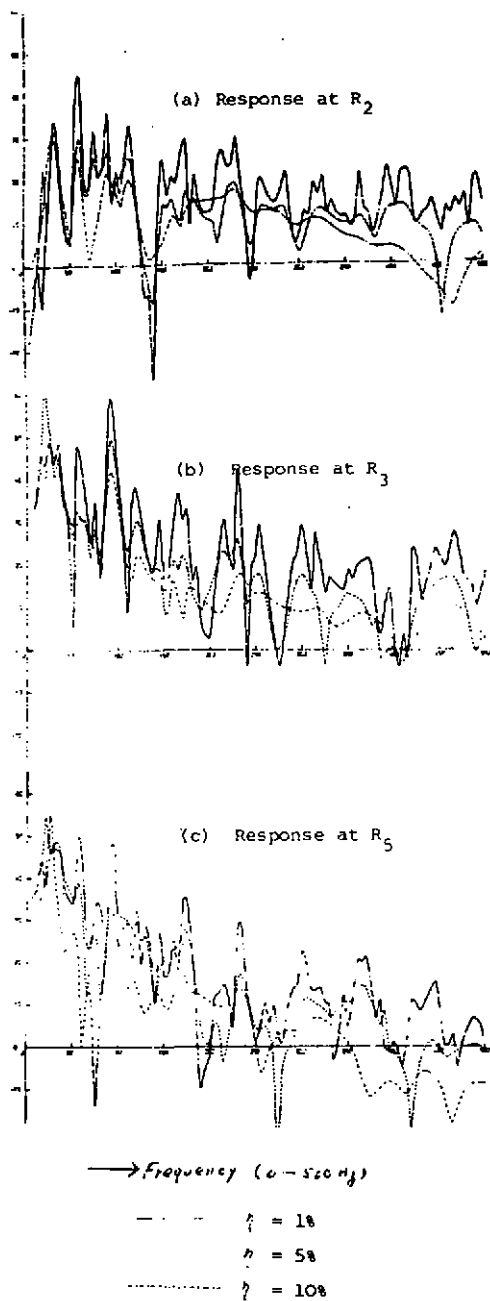


Fig.4